

RESEARCH PAPER

OPEN ACCESS

Modeling of uniaxial compressive strength by genetic programming and neuro-fuzzy

Dadkhah Rasool, Madani Esfahani Nasser¹, Hoseeinmirzaee Zahra^{*}

Young Researcher and Elite Club, Khorasgan (Isfahan) Branch, Islamic Azad University, Isfahan, Iran

¹Department of Mining Engineering, Faculty of Engineering, South Tehran Branch, Islamic Azad University, Tehran, Iran

Article published on August 28, 2014

Key words: Neuro-Fuzzy, Genetic Programming, Uniaxial Compressive Strength.

Abstract

Uniaxial Compressive Strength (UCS) is the most important rock parameter required and determined for rock mechanical studies in most civil and mining projects. In this study, two soft computing approaches, which are known as neuro-fuzzy inference system (ANFIS) and Genetic Programming (GP), are used in strength prediction of uniaxial compressive strength (UCS). Block Punch Index (BPI), porosity (n), P-wave velocity (Vp), Density () were used as inputs for both methods and were analyzed to obtain training and testing data. All of 130 data sets, the training and testing sets consisted of randomly selected 110 and 20 sets, respectively. Results showed that the ANFIS and GP models are capable of accurately predicting the uniaxial compressive strength (UCS) used in the training and testing phase of the study. The GP model results better prediction compared to ANFIS model.

*Corresponding Author: Hoseeinmirzaee Zahra 🖂 Zahra.mirzaee@gmail.com

Introduction

In the beginning of rock mechanics (in the early 1960s), more attention has been paid to the intact rock than to the other features of rock mass. The reason of it: First, the subject of it related heavily to the general mechanics of solid materials. Second, intact rock samples are obtained easily from drill cores.

The compressive strength is probably the most widely used and quoted rock engineering parameter. Under uniaxial load conditions the maximum stress that the rock sample can sustain referred as uniaxial compressive strength (σ ucs or σ c). The most useful description of the mechanical behavior of intact rock is the complete stress – strain curve of the compressive strength test. From this curve can be determined the Young modulus and the post-peak behavior of the rock material.

Rock material refers to intact rock discontinuities in the rock mass separated by the fracture. Uniaxial compressive strength (USC) of rock material usually used to classify Rock will assist. Analysis of rock mass strength parameters need to strong experimental and theoretical foundations. Measures and estimates of Uniaxial Compressive Strength (UCS) of rock materials are widely used in rock engineering; they are important for intact rock classification and rock failure criteria. In addition, analytical and numerical solutions require UCS. The procedure for measuring this parameter has been standardized by both the American Society for Testing and Materials (ASTM) and the International Society for Rock Mechanics (ISRM). High-quality core samples are needed for the application of UCS test in laboratories; a careful execution of this test is very difficult, time consuming, expensive and involves destructive tests. In order to overcome these difficulties, encountered during core sample preparation and execution of these tests, some predictive models considering simple index parameters such as Schmidt hammer, point load index, P-wave velocity and physical properties were developed by many investigators (Kahraman, 2001; Yilmaz and Sendir, 2002; Tsiambaos and Sabatakakis, 2004; Fener et. al. 2005). Because these indexes test require a relatively small number of samples, are quick and easy to execute, with portability and low costs, compared with uniaxial compressive strength tests. Despite some deficiencies, index tests, when coupled with experienced judgment, can provide initial estimates of rock properties, required at the feasibility and design stage (Yasar and Erdogan, 2004; Hanifi, 2009). Traditionally, statistical methods used in rock engineering, such as simple and multiple regression techniques are employed to establish predictive models (Dehghan, 2010). In recent years, new techniques such as genetic programming and fuzzy inference systems have been employed for developing predictive models to estimate the required parameters (Gokceoglu, 2002; Sonmez and et. al. 2004; Karakus and Tutmez, 2006; Yilmaz, 2007; Tiryaki, 2008).

The aim of this study is creative modeling of uniaxial compressive strength by genetic programming and neuro-fuzzy.

Materials and methods

Research Method

In this study, use for constructing the neuro network for prediction of uniaxial compressive strength. Various types of rock cores including Limestone, Hornfels, Travertine, Andesite, and Sandstone were gathered from different mine sites in Iran. A reliable predictive model requires a sufficiently large number of high-quality data. For this purpose 10 block samples were collected from the mine sites and 130 sample sets were obtained for rock mechanical tests. Followings the core retrieving, rock samples were prepared and some related laboratory rock tests such as Block Punch Index(BPI), porosity (n), P-wave velocity (Vp), Density (p), uniaxial compressive strength (UCS) were carried out in accordance with ISRM. The basic descriptive statistics of the dataset according to the rock type and data are summarized in Table 1.

J. Bio. & Env. Sci. 2014

Rock	Case	e	ρ(gr/cr	n ³)	n(%	5)	$v_p(k$	m/s)	B	PI		UCS(MPa))
Туре	#	Max	$\overline{x} \pm$	Min	Max	$\overline{x} \pm$	Min Max	$\overline{x} \pm$	Min Max	$\overline{x} \pm$	Min Max	$\overline{x} \pm$	Min
limestone	26	2.722	2.61 <mark>±</mark> 0.0	82.45	4.91	2.14±1.5	0.2949.83	38.61 ± 6.13	25.8733.01	2.26 <mark>±</mark> 6.48	12.16 173.76	96.25 <mark>±</mark> 32.31	34.78
Andesite	26	2.72	2.60 <mark>±</mark> 0.09	2.35	16.87	7.05 <mark>±</mark> 4.83	0.2949.83	31.68 <mark>±</mark> 7.89	16.69 38.1	2.05 <mark>±</mark> 11.1	3.01 173.76	53.01 <mark>±</mark> 40.32	9.5
Hornfels	26	2.812	2.75 <mark>±</mark> 0.0	22.68	1.2	0.43 <mark>±</mark> 0.25	0.0957.76	43.21 <mark>±</mark> 2.51	37.9358.36	31.43 <mark>±</mark> 13.6	11.85335.82	264.7 <mark>±</mark> 55.96	133.58
Sandstone	e 26	2.822	2.23 <mark>±</mark> 0.2	25 1.7	37.69	17.91 <mark>±</mark> 8.8	6.51 52.02	32.95 <mark>±</mark> 6.57	21.56 10.99	3.19 <mark>±</mark> 2.7	0.27 99.72	39.3 <mark>±</mark> 29.12	5.33
Travertine	e 26	2.53	2.04 <mark>±</mark> 0.06	2.27	14.21	9.03 <mark>±</mark> 3.61	3.12 53.15	49.76 <mark>±</mark> 2.14	44.2920.82	11.44 <mark>±</mark> 4.04	4.96 99.72	52.83 <mark>±</mark> 19.00	23.26

Table 1. Basic descriptive statistics of the established dataset according to the rock type.

 \bar{x} + refers to average values with standard deviation.

Research Hypotheses

The result of this study is to investigate the usability of neuro-fuzzy inference system (ANFIS) and genetic programming (GP) in predicting the uniaxial compressive strength (UCS) by use five rock types and make comparison of prediction levels between developed models by using the related prediction values and results. The ANFIS and GP approaches were used to predict the uniaxial compressive strength (UCS). Complex relationship between the parameters affecting the UCS can be easily modeled by use ANFIS and GP approach unlike statistical models. Experimental UCS data were collected from various samples to be included in training and testing phase of ANFIS and GP approaches.

Results

In this study, it was basically aimed to explore the applicability of the GP and ANFIS for prediction of the UCS value of some rocks that have great significance for rock mechanics and foundation engineering. This section comparatively presents the analyses results obtained from these approaches and quantitative assessments of the model's predictive abilities. Of the 130 data sets, 110 were used for training the models and 20 which are not used in training stage were presented for testing of the models. In order to find out how accurate the results of the developed models are, a statistical verification criteria was utilized as coefficient of correlation (R). As can be seen in Table 4 the comparisons between GP and ANFIS indicate that the best results in terms

of the R value generated from the GP analyses that are shown in (Fig. 3, 4) this implies that GP models produce good performance. In statistics, the overall error performances of the relationship between two groups can be interpreted from the R values. If a proposed model gives R > 0.8, there is a strong correlation between measured and predicted values overall the data available in the database.



Fig. 1. Expression Tree.



Fig. 2. First order TS model real Legend:



Fig. 3. Predicted UCS by ABFIS model vs. measured UCS for testing set.

Discussion

Genetic Programming

proposed genetic programming (GP) technique which is an extension to Genetic algorithms. In genetic programming, populations of hundreds or thousands of computer programs are genetically bred. This breeding is done using the Darwinian principle of survival and reproduction of the fittest along with a genetic recombination (crossover) operation appropriate for mating computer programs [Koza, 1992]. GP breeds computer programs to solve problems by executing the following three steps: (1) Generate an initial population of random computer programs composed of the primitive functions and terminals of the problem. (2) Iteratively perform the following sub-steps until the termination criterion is satisfied: (a) Execute each problem in the population so that a fitness measure indicating how well the program solves the problem can be computed for the program. (b) Create a new population of programs by selecting programs in the population with a probability based on fitness and then applying the following primary operations:

(i) Reproduction: Copy an existing program to the new population.

(ii) Crossover: Create new computer programs by crossover.

(iii) Mutation: Create new computer programs by mutation.

(iv) Choose an architecture-altering operation to one selected program.

(3) The single best computer program in the population produced during the run (best solution so far) is designated as the result of genetic programming (Kayadelen, 2009; Togun and Baysec, 2010).

GP model development

An aim of this study is to obtain an explicit formulation for Uniaxial Compressive Strength (UCS) using genetic programming based on experimental results. Details of the experimental procedure have been explained in Section 2. The details of the experimental database including the parameters and their range are presented in Table 3. To achieve generalization capacity for the formulations, the experimental database is divided into two sets as training and test sets. The formulations are based on training sets and are further tested by test set values to measure their generalization capability. In the literature, this type of studies includes test sets as 20– 30% of the training set. The patterns used in testing and training sets are selected randomly. Among the experimental data, 110 sets were used for GP training and 20 sets for GP testing. Parameters of the GP models are presented in Table 2. The purpose of this

It should be noted that proposed GP formulations in Eq. (1) is valid for the ranges of training set given in Table 3.

Table 2. Parameters of the GP model.

Population size	50
Maximum number of	1000
evaluated individuals	
Maximum depth	14
Reproduction	0.1
Initial prob stype	fixed
Num back gen	3
Probability of crossover	0.02
Probability of mutation	0.97
Percent change	0.25
Function set	+, -, *, /, power,
	exp, ln(x), log, p,
	$X^2, X^3, (1/X).$

Table 3.	Variables	used in 1	model (construction.
----------	-----------	-----------	---------	---------------

variables	code	range
Density	X1	1.72-2.82
Porosity	X2	0.09-37.69
Wave Length	X3	1669.84-5776.21
Box-Punch Index	X4	0.27-58.36
UCS	-	5.33-335.82

Table 4. Coefficients of correlation obtained for thepredictions made by ANFIS and GP.

section is to obtain the explicit formulation of Uniaxial Compressive Strength (UCS) as a function of Block Punch Index(BPI), porosity (n), P-wave velocity (Vp), Density (ρ). Explicit formulations based on GP for UCS was obtained as a function of experimental parameters as

UCS = $f(BPI, n, v_p, \rho)$

(Fig. 1) shows the expression tree of GP models, whose explicit formulation is:

$$\leq 4e^{\cos\left(\log\left(\left(\left(\rho\sin\left(\frac{\pi}{2}\right)\times 3\right)\right)\right)} + BPI^{d}$$
(1)
UCS

UCS	ĸ
GP	0.96
ANFIS	0.87

Neuro-fuzzy inference system (ANFIS)

In classical set theory, there is a crisp definition as to whether a variable belongs to a set or not. However, the fuzzy theory introduced does not give a sharp answer to questions. In this approach, the belongings of a variable to different sets are defined partially by continuous membership functions that vary between 0 and 1 (Dubois and Prade, 1980; Topcu and Saridemir, 2008). Mamdani and Tagagi-Sugeno (TS) models are two types of fuzzy approach commonlyused (Takagi and Sugeno, 1985). The main difference between these approaches is that Mamdani model uses the human expertise and linguistic knowledge to design the membership functions and if-then rules whereas TS model uses optimization and adaptive techniques to establish the system modeling and also uses less number of rules. TS model preferred mostly for mathematical analysis and its computational efficiency seems to be more advantageous than Mamdani model (Tutmez and Tercan, 2007). Also, the output membership function in TS model is simply designed as either linear or constant (Shahin et. al. 2003). Jang (1993) proposed a new fuzzy logic model called ANFIS which uses learning and parallelism properties of artificial neural network (ANN).

Fuzzy rules and membership functions are also generated adaptively by neural training process using given data set. So, ANFIS employs method of grid partitioning and subtractive clustering (Demuth and Beale, 2001; Padmini *et. al*, 2008; Aytac Guven *et. al*, 2009). First-order Sugeno type fuzzy inference system is used for linear function and zero-order Sugeno type fuzzy inference system is used for constant function. A typical two if then rules used in first-order Sugeno type is given in the following form:

If
$$x = A_1$$
 and $y = B_1$ then $f_{1(x,y)} = p_1 x + q_1 y + k_1$ (2)

If
$$x = A_2$$
 and $y = B_2$ then $f_{2(x,y)} = p_2 x + q_2 y + k_2$
(3)

where x (or y) is *ith* input node, p, q and k are training parameters, A and B are fuzzy membership function labels.

The membership function is updated by backpropagation learning algorithm (Gray, 1998). The basic structure of an ANFIS model is shown in (Fig. 2). As can be seen there are five layers in which the mathematical computations are performed. The mathematical computations in fuzzy approach are performed in five stages. The value of the *ith* node of the first stage is computed as below;

$$U_{1,i} = \eta A_i(x) \text{ for } i = 1,2 \text{ or }$$
(4)

$$U_{1,i} = \eta B_{i-2}(x) \text{ for } i = 3,4$$
(5)

where η is the membership function.

In second stage, the nodes are represented as the fire strength of the rule and the output $U_{2,i}$ which is the product of the incoming signals is computed as follow;

$$U_{2,i} = w_i == \eta A_i(x) \eta B_i(y), \quad i = 1,2$$
(6)

In third stage, the normalized firing strengths which shows the ratio of the ith rule's firing strength versus all rule's firing strength are computed by following equation;

$$U_{3,i} = U_{3,i} = \overline{w}_i = \frac{w_i}{w_1 + w_2}$$
(7)

The subsequent stage performs a calculation for determination of the contribution of the *ith* rule to output;

$$U_{4,i} = \overline{w}_i f_i = \overline{w}_i (p_i x + q_i y + k_i)$$
(8)

 \overline{w} indicates the normalized firing strength found from layer **3**, p_i , q_i and k_i are the consequent parameters. In last stage, the final output of the ANFIS is computed by following the equation;

$$U_{5,i} = \sum_{i} \overline{w}_{i} f_{i} = \frac{\sum_{i} w_{i} f_{i}}{\sum_{i} w_{i}}$$
(9)

Development of ANFIS model

ANFIS model was developed using identical inputs for as in GP, for generation of the membership functions associated with each input variable, the grid partition method was employed for ANFIS model. In the model, the Gaussian membership function was assigned. The hybrid learning algorithm was used for optimizing the parameters allows a fast identification of parameters and substantially reduces the time needed to reach convergence (Mehmet *et. al.* 2010). The minimum validation error is used as the stopping criterion to avoid over fitting. The ANFIS model has 80 linear parameters, 24 nonlinear parameters, 55 nodes and 16 fuzzy rules. The MATLAB Software was used for the models development.

Conclusions

This study demonstrates the efficiency of GP and ANFIS models to predict UCS. The developed models were able to predict the UCS for Block Punch Index(BPI), porosity (n), P-wave velocity (Vp), Density (ρ) used in training and testing processes. Predicting of UCS as a function of parameters is a difficult task to achieve. However, a successfully trained GP and ANFIS models can predict the UCS easily and accurately. So, the GP and ANFIS models can be a powerful alternative approach to traditional statistical methods used in developing the

relationship between the UCS and the parameters affecting it. Although the performance of the developed GP and ANFIS models is limited to the range of input data used in training process, the model can easily be retrained to expand the range of input variables by providing additional new set of data. GP and ANFIS models also have the minimum degree of scatter and maximum ability of trend capture compared to other equations. But as mentioned in section five, the GP model in the paper results better prediction compared to ANFIS model. We believe that genetic programming based techniques will gain much more popularity for strength prediction applications in the literature and applications in the future.

References

Guven A. Azamathulla HM, Zakaria NA. 2009. Linear genetic programming for prediction of circular pile scour. Ocean Engineering **36**, 985–991.

Kayadelen C, Günaydın O, Fener M, Demir A, Zvan A. 2009. Modeling of the angle of shearing resistance of soils using soft computing systems. Expert Systems with Applications **36**, 11814 -11826.

Dehghan S. 2010. Prediction of uniaxial compressive strength and modulus of elasticity for Travertine samples using regression and artificial neural networks. Journal of Mining Science and Technology **20**, 41–46.

Demuth H, Beale M. 2001. Neural network toolbox for use with MATLAB. The MathWorks Inc.

Dubois D, Prade H. Fuzzy, 1980. Sets and systems. New York: Academic Press.

Fener M, Kahraman S, Bilgil A, Gunaydin O. 2005. A comparative evaluation of indirect methods to estimate the compressive strength of rocks. Journal of Rock Mechanic. **38(4)**, 329.

Gokceoglu C. 2002. A fuzzy triangular chart to predict the uniaxial compressive strength of Ankara agglomerates from their petrographic composition. Journal of Engineering Geology, 66 (39).

Gray G, Murray-Smith D, Sharman K, Weinbrenner T. 1998. Nonlinear model structure identification using genetic programming. Control Eng Pract; **6**, 1341–52.

Hanifi C, 2009. anakcı. Prediction of compressive and tensile strength of Gaziantep basalts via neural networks and gene expression programming. Neural Comput & Applic **18**, 1031–1041.

Jang J. 1993. ANFIS: adaptive-network-based fuzzy inference systems. IEEE Trans Syst Man Cybern; 23(3), 665–85.

Kahraman S. 2001. Evaluation of simple methods for assessing the uniaxial compressive strength of rock. Journal of Rock Mechanic, **38**, 981.

Karakus M, Tutmez B. 2006. Fuzzy and multiple regression modeling for evaluation of intact rock strength based on point load, schmidt hammer and sonic velocity. Journal of Rock Mechanic, **39(1)**, 45.

Koza J. 1992. Genetic programming: on the programming of computers by means of natural selection. Cambridge, MA: MIT Press.

Mehmet M, Kayadelen C. 2010. Modeling of transfer length of prestressing strands using genetic programming and neuro-fuzzy. Advances in Engineering Software **41**, 315–322.

Togun N, Baysec S. 2010. Genetic programming approach to predict torque and brake specific fuel consumption of a gasoline engine. Applied Energy. ARTICLE IN PRESS. **Padmini D, Ilamparuthi K, Sudheer K.** 2008. Ultimate bearing capacity prediction of shallow foundations on cohesionless soils using neurofuzzy models. Computer Geotechnic; **35**, 33–46.

Shahin M, Maier H, Jaksa M. 2003. Settlement prediction of shallow foundations on granular soils using B-spline neurofuzzy models. Computer Geotechnic; **30**, 637–47.

Sonmez H, Tuncay E, Gokceoglu C. 2004. Models to predict the uniaxial compressive strength and the modulus of elasticity for Ankara Agglomerate. Journal of Rock Mechanic, **41**, 717.

Takagi T, Sugeno M. 1985. Fuzzy identification of systems and its applications to modeling and control. IEEE Trans Syst Man Cybern;**15**, 116–32.

Tiryaki B. 2008. Predicting intact rock strength for mechanical excavation using multivariate statistics, artificial neural networks and regression trees. Journal of Engineering Geology, **99**, **51**.

Topcu IB, Saridemir M. 2008. Prediction of rubberized concrete properties using artificial neural network and fuzzy logic. Constraction Build Material; **22**, 532–40. **Tsiambaos G, Sabatakakis N.** 2004. Considerations on strength of intact sedimentary rocks. Journal of Engineering Geology, **72**, 261.

Tutmez B, Tercan A. 2007. Spatial estimation of some mechanical properties of rocks by fuzzy modeling. Journal of Computer Geotechnic;**34**, 10–8.

Yasar E, Erdogan Y. 2004. Correlating sound velocity with the density, compressive strength and Young's modulus of carbonate rocks. Journal of Rock Mechanic, **41**, 871.

Yilmaz I, Sendir H. 2002. Correlation of schmidt hardness with unconfined compressive strength and young modulus in gypsum from Sivas (Turkey). Journal of Engineering Geology, **66**, 211.

Yılmaz I, Yuksek A. 2007. An example of artificial neural network (ANN) application for indirect estimation of rock parameters. Journal of Rock Mechanic, **41(5)**, 781.